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[A new low carbon cementitious binder for stabilising weak ground conditions through deep soil mixing.](#)

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2 **A new low carbon cementitious binder for stabilising weak ground conditions**
3 **through deep soil mixing**

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5 P. Sargent*¹, P. N. Hughes² and M. Rouainia³

6

7 * Corresponding author

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9 1 AECOM, First Floor, One Trinity Gardens, Broad Chare, Quayside, Newcastle upon Tyne, NE1 2HF, UK.

10 2 Christopherson Building, School of Engineering and Computing Sciences, Durham University, South Road,
11 Durham, DH1 3LE, UK.

12 3 Drummond Building, School of Civil Engineering and Geosciences, Newcastle University, Newcastle upon
13 Tyne, Tyne and Wear, NE1 7RU, UK.

14

15

16 Paul Sargent MSci MSc PhD FGS

17 E-mail: paul.sargent@aecom.com

18 Tel: +44(0)7815 714237

19

20 Paul N. Hughes BSc MSc PhD

21 E-mail: paul.hughes2@durham.ac.uk

22 Tel: +44(0)191 334 2450

23

24 Mohamed Rouainia Dipl. -Ing MSc PhD

25 E-mail: mohamed.rouainia@newcastle.ac.uk

26 Tel: +44(0)191 208 3608

27

28 **Abstract**

29 Soft alluvial soils present unfavourable conditions for engineering developments, due to their poor bearing
30 capacities and high potential for experiencing shrinkage and swelling. This paper focusses on deep dry soil mixing
31 (DDSM), which introduces cementitious binders to soft soils via a rotating auger drill; thereby producing soil-
32 cement columns. Ordinary Portland cement (CEM-I) is globally used across the construction industry and is the
33 most commonly used binder for DDSM applications, due to its high strength performances. However, CEM-I
34 production is one of the world's most energy intensive and expensive industrial processes; contributing 5-7% of the
35 world's total CO₂. There is now significant pressure on the cement and construction industries to greatly reduce
36 their CO₂ emissions by developing "greener" alternatives to CEM-I, which are both more environmentally and
37 financially sustainable in the long-term. Alkali activated industrial by-products (IBP's) such as ground granulated
38 blast furnace slag (GGBS), known as geopolymers have been identified as potential alternatives. These are
39 advantageous due to negating the need to transfer IBP's to landfill, their abundance, negligible or zero production
40 costs. Geopolymers are capable of reducing greenhouse gas emissions by up to 64%. Calcium-bearing slags have
41 also been found to possess potential for carbon capture and storage (CCS). Comparisons with the strength and
42 durability of untreated and stabilised soils have been made in this study. Results indicate that stabilising an alluvial
43 soil with sodium hydroxide (NaOH) activated GGBS produced significant strength and durability improvements
44 surpassing CEM-I. The addition of NaOH allowed pozzolanic reactions to occur, leading to improved mechanical
45 properties with time; primarily strength.

46
47 **Keywords**

48 Deep dry soil mixing, sustainability, low carbon, alkali activation, performance.

49
50

51 **1.0 Introduction**

52 Soft soils including alluvium present problematic ground conditions, given their poor bearing capacity,
53 shrinkage/swelling, settlement and durability properties. Various ground improvement techniques may be adopted
54 to enhance the engineering performance of such soils, whereby the most appropriate technique depends on the
55 physico-chemical properties of the soil in question. Deep dry soil mixing (DDSM) is becoming an increasingly
56 popular and effective ground improvement technique in the UK for treating such soils by creating cemented soil
57 columns via auger mixing. Since the development of the DDSM technique, lime and Ordinary Portland Cement
58 (CEM-I) have traditionally been used as the binders, given their abilities to produce impressive strengths -
59 particularly for CEM-I. The presence of soil water and calcium silicates/aluminates within the cementitious binders
60 injected into the soil during mixing react to form hydration products including calcium silica hydroxide (C-S-H)
61 and calcium aluminate hydroxide (C-A-H) gels. For DDSM, the physico-chemical properties of a soil to be
62 considered in selecting the most appropriate cementitious binder include particle size distribution, plasticity, pH,
63 moisture content, cation exchange capacity (CEC), specific surface area, organic and sulphate contents. For lime
64 and cement stabilisation to work effectively; low organic contents (<1%), low sulphate contents (<0.3%) and clay
65 contents of 10 – 50% are required (Tutumluer, 2012).

66 Unfavourable environmental and financial issues are associated with utilising CEM-I as a binder. Cement
67 production contributes 5 – 7% of global CO₂ emissions (McLellan et al., 2011). Increases in such emissions are
68 anticipated to have long term adverse warming effects on the global climate; the consequences of which include
69 changing weather patterns, melting polar ice caps, rising sea levels and ocean acidification (EPA, 2015). These
70 effects will have potentially major impacts on marine and land based life. Therefore, it has become extremely
71 important for the construction industry to become more sustainable by using materials with lower values of
72 embodied CO₂. The continued use of CEM-I is also financially unsustainable, whereby its manufacture is energy
73 intensive and expensive. The electrical energy consumed per ton of CEM-I produced is 75 kWh (Madlool et al.,
74 2011). The UK's price indices of electricity and coal as fuels rose by 75 and 63% respectively between 2005 and
75 2011. Similar trends were observed during the same period in the United States, whereby electricity and coal prices
76 increased by 19 and 47%, respectively (Imbabi et al., 2012). Since the Fukushima nuclear disaster in 2011, global
77 coal prices have reduced by half to approximately \$70 per ton (Reuters, 2015), as have oil prices from \$120 to
78 approximately \$45 per barrel (LSE, 2015). However, according to DECC (2015) and EIA (2015), electricity prices
79 in the UK and US have remained relatively constant since 2011.

80 Hence, there is a need to identify more environmentally and financially sustainable replacement binders. These
81 binders should satisfy the following criteria in that they: 1) provide engineering performances comparable to or
82 surpass those of CEM-I within similar curing times, 2) be commercially available at comparable/lower prices than
83 CEM-I, 3) produced in such a way that fuel costs are markedly lower than those affiliated with CEM-I production
84 whilst incurring lower CO₂ emissions. A modern popular route for selecting new binders has been to recycle
85 alumino-silicate based pozzolanic industrial by-products (IBP's) such as ground granulated blastfurnace slag
86 (GGBS) and pulverised fly ash (PFA). Such materials are already being used as additives to cement mixtures;
87 namely CEM-II for PFA and CEM-II/III for GGBS. Per Hanson (2014), GGBS is preferred over PFA in the UK as
88 a replacement for CEM-I in cement mixtures due to its higher levels of replacement and ability to produce higher
89 strengths compared with PFA. However, the aim of this research area is to design new low carbon cementitious
90 binder mixtures which negate the need to use any lime or CEM-I.

91 Laboratory and field-based research by workers including Hughes and Glendinning (2004) and Sargent et al.
92 (2013) have demonstrated that GGBS has significant potential as a sustainable replacement for lime and CEM-I;
93 whereby Hughes and Glendinning (2004) implemented GGBS for stabilising peaty soils through DDSM along the
94 Channel Tunnel Rail Link. However, there are instances when soil pH is too acidic and moisture contents are too
95 high for strength gains to develop when just using GGBS. The hydraulicity of the GGBS is latent - i.e. confined
96 within its glassy structure (Newman and Choo, 2003). Hence, the addition alkali-activators aims to release the
97 GGBS reactivity by raising soil pH and ultimately increase rates at which the mechanical properties of stabilised
98 soils are improved (Palomo et al., 1999). Once the stabilised soil's pH reaches 10.5 (Davidson et al., 1965), the
99 GGBS reactivity has been triggered and will start to react over long periods of time with the soil water via
100 pozzolanic reactions to produce cementitious gels. Such materials are known as "Geopolymers".

101 The continued use of lime as an alkali activator is environmentally unfavourable due to their high carbon costs.
102 According to Shi et al. (2006), solid NaOH flakes/pellets and Na₂SiO₃ solution are becoming two of the most
103 widely available and popular activators for use in concretes and cements. Cristello et al. (2011) used NaOH and
104 Na₂SiO₃ to activate IBP's (class F PFA) for use in DDSM treatment of a low plasticity sandy clay. Field testing
105 results demonstrated that the activated geopolymer produced higher strengths compared with CEM-I.

106 Per Habert et al. (2011) lower financial and environmental costs are associated with their production over lime and
107 CEM-I. However, NaOH flake/pellet production has lower environmental impacts (3.71 x 10⁻¹ kg CO₂ eq, human
108 toxicity level of 15.84 kg 1,4-DB eq and a freshwater ecotoxicity of 3.98 kg 1,4-DB eq) compared with Na₂SiO₃

109 solution, which has a high global warming potential of 117.8 kg CO₂ eq, high human toxicity level of 82.75 kg 1,4-
110 DB eq and a high freshwater ecotoxicity of 21.84 kg 1,4-DB eq.

111 Further justification behind using NaOH over Na₂SiO₃ as an activator is that the latter has a higher accelerated
112 carbonation depth when used within geopolymer pastes, which has been attributed to the composition and structure
113 of the C-S-H gels formed; whereby NaOH activated slags have a higher Ca/Si ratio (1.2) compared with Na₂SiO₃
114 activated slags (Bernal, 2014). However, with NaOH activated slags possessing such a higher Ca/Si ratio and
115 reduced silicate chain length, these properties may consequently favour the precipitation of increasing quantities of
116 carbonates to fill pore spaces. The MgO content of slags has been identified to have a significant role in the
117 mechanism and extent of carbonation; whereby slags with higher MgO content are more likely to be less effected
118 by carbonation. Thus, on this basis and the typical MgO contents of various slag types per Sanna et al. (2012), the
119 extent to which carbonation extends within GGBS-based geopolymers is anticipated to be lower compared with
120 steel slags, argon-oxygen decarburisation (AOD) process slag and most ash wastes (e.g. PFA).

121 Under accelerated carbonation testing (i.e. >1%), if alkali-activated slag pastes show similar carbonation depths as
122 CEM-I pastes, it is considered likely by Bernal (2014) that the alkali-activated slag will be much more durable
123 under natural (atmospheric) carbonation conditions. C-S-H gels within alkali-activated slag mixtures are known to
124 be prone to turning into amorphous silicate gels post-carbonation, which consequently reduces the strength of the
125 material (Song et al., 2014). Whilst using higher dosages of alkali-activated slag binders significantly reduces the
126 carbonation depth (Bernal, 2014), simply adjusting the slag/alkali activator ratio of the binder in favour of the
127 activator causes an increased rate of reaction, produces more C-S-H and therefore increases strength and
128 carbonation resistance (Song et al., 2014). Experiments undertaken by Song et al. (2014) demonstrated that by
129 changing the GGBS/alkali activator ratio from 94/6 to 86/14, this increased 28 day compressive strengths (no
130 carbonation) from 14.9 to 42.5MPa, increased 21 day carbonated compressive strengths from 7.6 to 28.9MPa and
131 reduced carbonation depth from 25 to 10mm – which is typical for pastes using CEM-I.

132 This paper investigates whether NaOH can successfully activate GGBS to ultimately produce engineering
133 performances similar to CEM-I and lime when incorporated within a soft alluvial soil whilst minimising
134 carbonation potential, and whether the binder is suitable for use in DDSM treatment.

135

136

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138 **2.0 Soil and Binder Materials**

139 2.1 Lanton Alluvium

140 For this paper, an alluvial soil was sourced from the flood plain of the River Glen in Lanton, 4km north west of
141 Wooler in Northumberland, UK. Disturbed and undisturbed U100 samples from depths of 1.5 – 2.4m were
142 obtained.

143
144 2.2 Cementitious Binder

145 The IBP binder used for this research was ground granulated blast-furnace slag (GGBS) supplied by Hanson
146 Cements Ltd. A particle size distribution (PSD) curve for the GGBS is provided within Figure 1. For comparison
147 purposes, non-activated GGBS, CEM-I (supplied by Lafarge) and lime (supplied by Fisher Scientific UK Ltd) were
148 also used. To ensure that the binder to be utilised for stabilising Lanton alluvium in this paper had low embodied
149 carbon in terms of energy and production costs, lower financial cost and reduced potential carbonation depth, the
150 Na₂SiO₃ solution activator was not used. Thus, solid NaOH pellets were used as an activator, which was supplied
151 by Fisher Scientific UK Ltd.

152

153

Table 1: Summary of the GGBS properties. Courtesy of Hanson (2016)

Property	Value
Granulated grain size (mm)	0-5
Colour	Light grey
Odour	N/A
pH (DEV-S4-eluate according EN 12457-4)	10 - 12
Density at 20°C (Mg/m ³)	2.4-3.0
Water solubility (g/l)	<1
Melting/freezing point (°C)	>1000
Flash point	N/A (inorganic)
<u>Composition:</u>	
EG-Nr.	266-002-0
CAS-Nr.	65996-69-2
CaO (%)	40
SiO ₂ (%)	35
Al ₂ O ₃ (%)	12
MgO (%)	10
Fe ₂ O ₃ (%)	0.2

154

155 **3.0 Methodologies**

156 3.1 Soil Index Properties

157 A series of geotechnical index tests were conducted according to BS 1377 (BSI, 1990). These included PSD
158 through wet sieving and sedimentation (Figure 1), Atterberg limits (LL = 35.7%, PL = 20.7%, PI = 15.0), particle
159 density (2.61Mg/m³), natural in-situ moisture contents (25%) and compaction testing to determine the soil's
160 optimum moisture contents (14.7%), bulk (2.0Mg/m³) and dry (1.74Mg/m³) densities. According to BS 5930

161 (2015), Lanton alluvium may be categorised as a silty SAND in terms of dry density. A summary of these
162 geotechnical index properties is provided within Table 2.

163 Modern day loadings imposed on geotechnical structures and foundation soils are generally complex, comprising a
164 mixture of monotonic, cyclic and dynamic loadings occurring at different magnitudes over various timescales. With
165 reference to the cyclic and dynamic loads, these may be imposed by human-related activities such as high-speed
166 trains and highway loads and/or natural phenomena such as earthquakes. Depending on the soil type, moisture
167 content and loading conditions, both human-related activities and earthquakes can cause soil liquefaction, which if
168 occurs can severely compromise the integrity of geotechnical structures, along with any other surrounding
169 infrastructure. Thus, the potential of the Lanton alluvium to undergo liquefaction upon cyclic/dynamic loading was
170 assessed according to Japanese criteria defined by Tsuchida (1970) for port and airport design. This criteria was
171 selected on the basis that Japan is a seismically active region with a well established background into understanding
172 the liquefaction behaviour of soils and the remediation measures available. Additionally, the UK does not currently
173 have any such comparable British Standards whereas ASTM only has a standard which evaluates a soil's
174 liquefaction potential through the normalisation of penetration resistance specifically for sands.

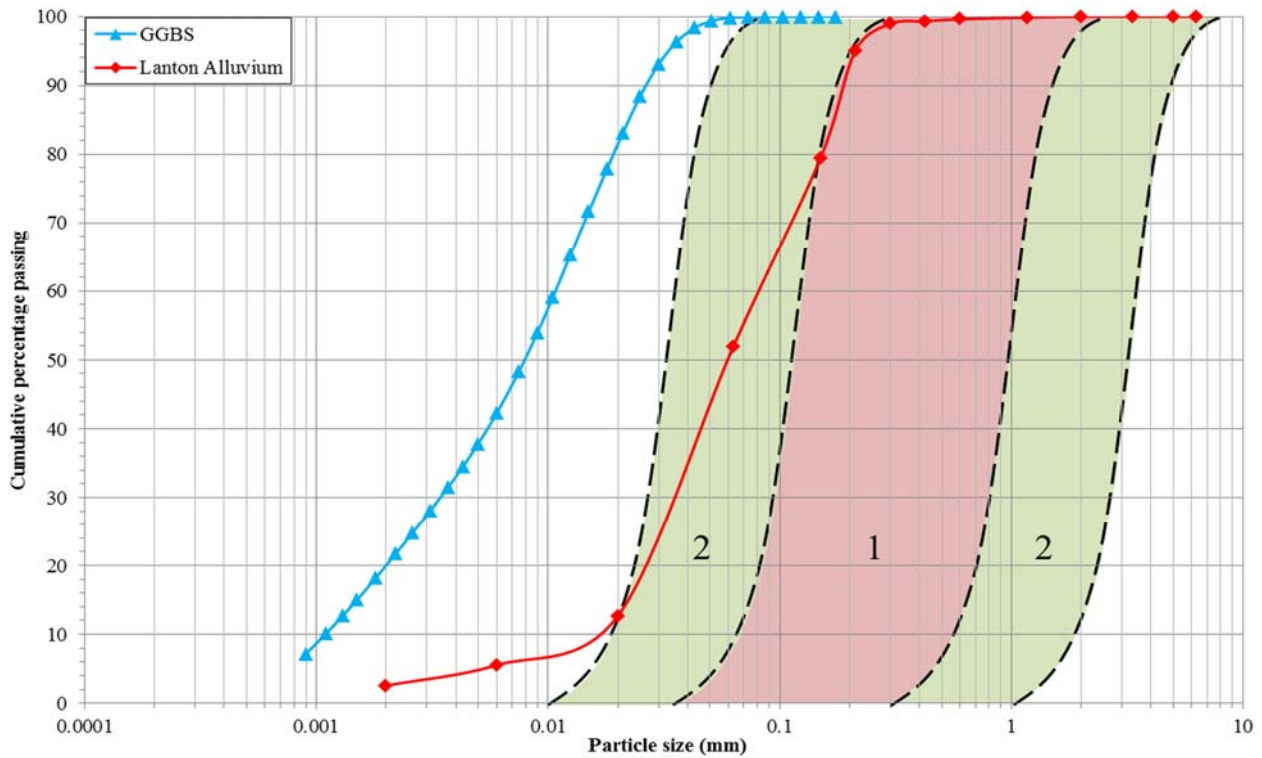
175 According to Tsuchida's (1970) soil grading criteria, the Lanton alluvium's PSD curve (Figure 1) lies within the
176 zone for soils which may potentially liquefy. This raises concerns about the soil's strength, thereby emphasising the
177 need for treatment to ensure that the ground conditions are suitable for future engineering developments,
178 particularly those involving variable cyclic and dynamic loadings.

179 The soil was also subjected to physico-chemical tests to understand their capacities to react with binders and
180 produce cementitious gels. The pH of the soil in its untreated and stabilised states after various curing periods were
181 measured per BS 1377 part 3 (BSI, 1990). CEC and specific surface area testing were also conducted. The former
182 provides an estimate of the number of sites on clay minerals where cation exchange may occur. Soils with high
183 surface area values (i.e. higher clay contents) result in higher CEC values. These properties provide insights into
184 the soil's potential for undergoing cementitious reactions. The method used for CEC analysis was BS 7755 (1995).
185 The CEC-related charge density (σ_{CEC}) for the soil was obtained as follows:

$$186 \quad \sigma_{CEC} = \frac{e (CEC \times 10^{-2})}{2 ab}$$

187 where e is the elementary charge (1.6022×10^{-2} C), a and b are unit cell parameters for clay minerals in the x-y
188 plane (Meunier, 2005). The soil had an average CEC of 11.45cmol/kg and σ_{CEC} of 0.0019.

189



190

191 **Figure 1:** PSD curves for Lanton alluvium soil (red) and GGBS (blue) with Tsuchida (1970) liquefaction criteria superimposed
 192 (red area “1” denotes zone for most liquefiable soils, green area “2” denotes zone for potentially liquefiable soils).

193

194 The Brunauer Emmett Teller (BET) nitrogen absorption method (Brunauer et al., 1938) was adopted to determine
 195 the surface area of the Lanton alluvium by using a Micromeritics Tristar 300. An average surface area of 6.45m²/g
 196 was recorded for the soil. Total organic carbon (TOC) and total organic matter (TOM) testing revealed low values
 197 of 0.42% and 0.76%, respectively. Per Nair and Little (2009), sulphates within soils can have detrimental effects on
 198 their engineering performances when stabilised with cementitious binders. This arises from the likely formation of
 199 ettringite and/or thaumasite, which swell upon contact with water. The sulphate content of the soil was determined
 200 through the use of a Dionex Ion-Chromatography ICS-1000, which revealed a low sulphate content of 49 mg per
 201 kg. Finally, 100 mm diameter remoulded and undisturbed samples of Lanton alluvium were placed within a triaxial
 202 cell under isotropically consolidated conditions to determine their coefficients of vertical permeability (k_v), per BS
 203 1377 (BSI, 1990). At 22°C, the k_v values obtained for the soil in its undisturbed and remoulded states were 1.41 x
 204 10⁻⁸ m/s and 5.58 x 10⁻⁹ m/s, respectively.

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Table 2: Summary of the soil's index properties.

Property	Value
In-situ moisture content (%)	25.0
Plasticity Index	14.95
Liquid Limit (%)	35.66
Saturated unit weight (kN/m ³)	18.44
Bulk Density (Mg/m ³)	1.99
Dry density (Mg/m ³)	1.74
Cation exchange capacity (cmol/kg)	11.45
Specific surface area (m ² /g)	6.45
Total organic content (%)	0.76
Sulphate content (mg/kg soil)	49.0
BS 5930 classification ^[15]	Silty SAND

209

210

211

3.2 Engineering Performance Testing

212

3.2.1 Preparation of Testing Samples

213 Numerous binder combinations were added to the soil at dosages of 2.5 – 10% by dry weight to identify the most
 214 effective binder and corresponding dosage. Once soil samples were obtained from the field, they were oven dried at
 215 110°C for 24 hours and crushed into a fine powder (particle size ≤ 1 mm). This provided a starting point for
 216 achieving a specific moisture content of 25%, made experiments repeatable and also increased practicality when
 217 mixing the dry powdered binders within the soil in the laboratory. Stabilised samples were mixed via a rotary mixer
 218 for 10 minutes to maximise homogeneity and therefore the number of reactive sites for cementitious bond
 219 formation. Water was then added to the mixtures to achieve the required pre-treatment (in-situ) soil moisture
 220 content.

221 Samples were created by tamping and compressing the samples into split-sample moulds, which were then placed
 222 into a hydraulic press to compact the material to the required dimensions and the compacted conditions induced
 223 upon DDSM. For compressive strength tests, samples 38mm in diameter and 76mm long were formed. For
 224 oedometer testing, separate samples with dimensions of 76mm in diameter and 18mm in thickness were prepared.
 225 Finally, samples 105mm in diameter and 115mm in length were produced and cured for durability testing. All
 226 stabilised samples for each type of test were prepared with a bulk density of 1.9Mg/m³, based on optimum
 227 compaction criteria and a moisture content of 25%. Table 3 summarises the soil-binder mixtures examined and
 228 their respective concentrations.

229

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235 **Table 3:** Summary of the binder compositions used for the laboratory stabilisation of Lanton alluvium.

Soil – Binder Mixture	Composition
Lanton	Lanton (25% grav. water)
Lanton + 5% Lime	Lanton (25% grav. water) + 5% lime
Lanton + 10% Lime	Lanton (25% grav. water) + 10% lime
Lanton + 5% CEM-1	Lanton (25% grav. water) + 5% CEM-I
Lanton + 10% CEM-1	Lanton (25% grav. water) + 10% CEM-I
Lanton + 5% GGBS	Lanton (25% grav. water) + 5% GGBS
Lanton + 10% GGBS	Lanton (25% grav. water) + 10% GGBS
Lanton + 2.5% GGBS-NaOH	Lanton (25% grav. water) + 1.67% GGBS + 0.83% NaOH
Lanton + 5% GGBS-NaOH	Lanton (25% grav. water) + 3.33% GGBS+ 1.67% NaOH
Lanton + 7.5% GGBS-NaOH	Lanton (25% grav. water) + 5% GGBS + 2.5% NaOH
Lanton + 10% GGBS-NaOH	Lanton (25% grav. water) + 6.67% GGBS + 3.33% NaOH

236

237 For all compressive strength, durability, pH and moisture content testing, samples were cured within wax-sealed
 238 PVC sample moulds for periods of 0, 7, 14, 21, 28, 35, 42, 49 and 56 days and stored within a temperature
 239 controlled room (55% relative humidity, 20°C ambient air temperature). Whereas for compressibility testing,
 240 samples were prepared and cured within oedometer rings and tightly sealed using three layers of cling film. For
 241 compressibility and durability testing, samples were cured for 28 days before testing based on construction
 242 specifications by Hansson et al. (2001). Once cured, all samples were extruded, trimmed and tested.

243

244 3.2.2 Laboratory Tests

245 A suite of laboratory tests was conducted according to BS 1377 (BSI, 1990) to assess the performance of the
 246 various binders. Strength and stiffness were assessed via unconfined compressive strength (UCS). Samples were
 247 tested either to failure or a maximum axial strain of 15%. The compressibility of samples was assessed by
 248 oedometer testing, involving a series of load-unload-reload loops up to vertical stresses of 1600kPa. Per Davidson
 249 et al. (1965), pozzolanic reactions require a minimum soil pH of 10.5. Thus, the pH of all the stabilised mixtures
 250 was tested prior to and after each curing period. The durability of the stabilised mixtures was assessed by wetting-
 251 drying and freeze-thaw tests according to ASTM D559/559M and D560/560M (ASTM, 2015a,b), especially given
 252 that frequent wetting-drying can be expected in the UK's temperate maritime climate.

253

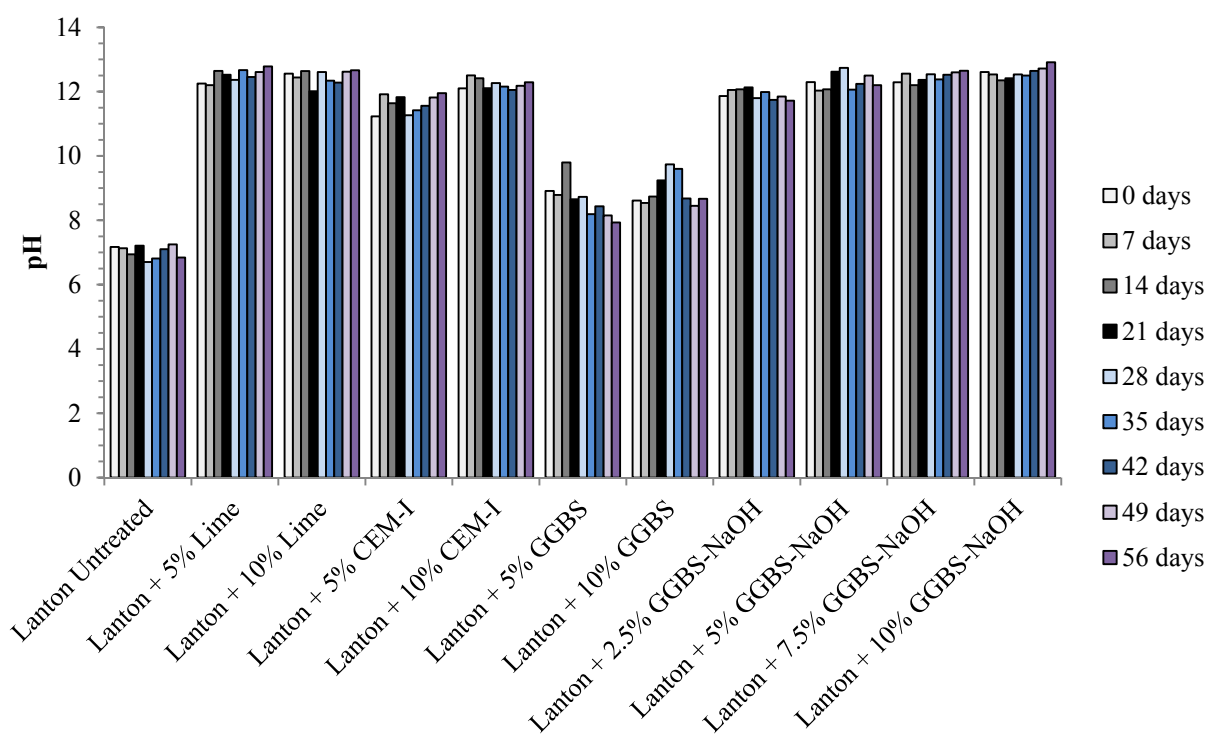
254 4.0 Results and Discussion

255 4.1 pH

256 Strength development of stabilised soils can be greatly influenced by their pH; whereby alkali activation may be
 257 required to raise soil pH to ≥ 10.5 to promote pozzolanic conditions. pH values recorded for Lanton alluvium and
 258 non-activated GGBS samples were all < 10 , highlighting the need for activation (Figure 2). For GGBS-NaOH
 259 specimens, all four dosages successfully raised soil pH to at least 11.7. The maximum pH values recorded ranged
 260 between 12.6 for the 7.5% dosage and 12.9 for 10%. These values were comparable with those recorded for

261 samples stabilised with 10% lime or 10% CEM-I; confirming that pH conditions were ideal for pozzolanic
 262 reactions.

263 Whilst stabilisation using lime and CEM-I at 5% raised and maintained a pH >10.5 for 56 days, poor strength and
 264 durability performances were recorded. A similar observation was made for 2.5% GGBS-NaOH samples, which
 265 may be attributed to: 1) insufficient binder dosage; 2) insufficient curing time for cementitious gels to form, 3) the
 266 soil's high water content and therefore low calcium concentration. Should oxidising reactions and subsequent pH
 267 reductions occur within stabilised samples, hydration and pozzolanic reactions may become inhibited and result in
 268 poor or severely delayed strength developments. Thus, it is essential to assess the quantities of activator required
 269 within samples to ensure that pH values >10.5 are maintained for the long-term.



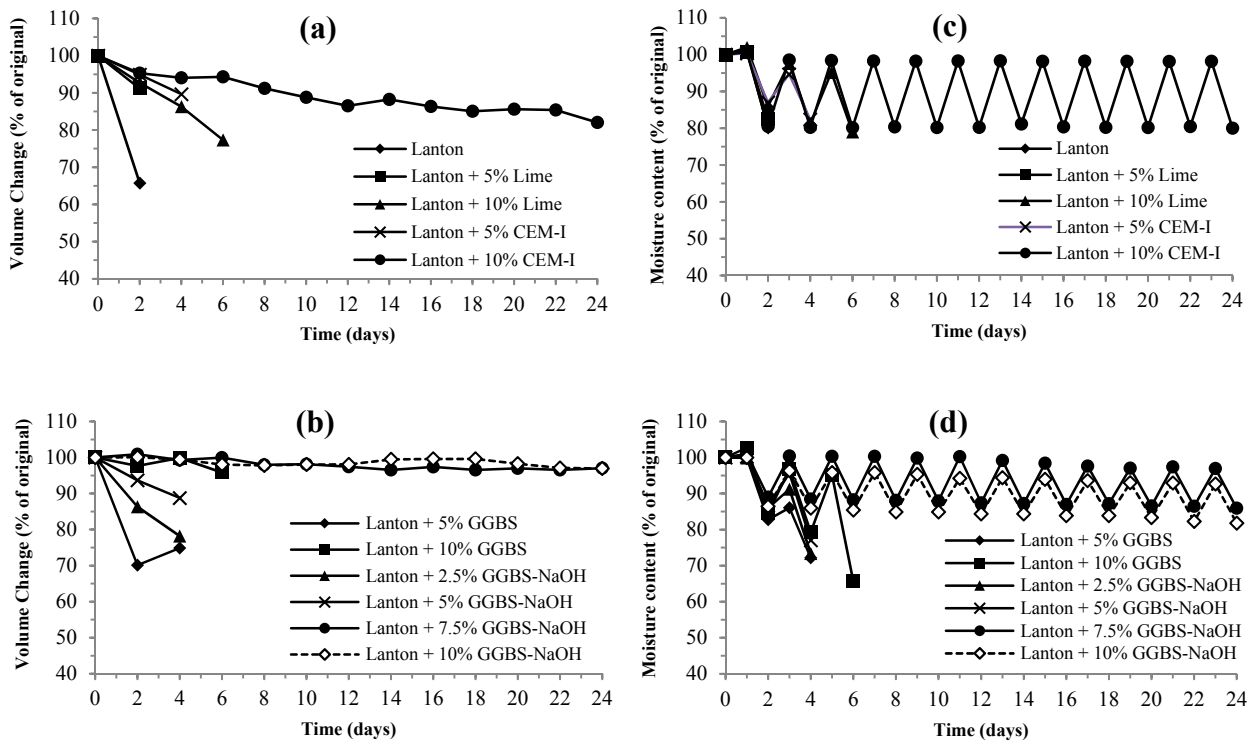
270
 271 **Figure 2:** pH for all binder mixtures and dosages used to stabilise Lanton alluvium over 56 days.

272
 273 **4.2 Durability**

274 **4.2.1 Wetting-drying**

275 All sample mixtures were subjected to wetting-drying durability testing to determine soil-cement loss and changes
 276 in moisture content and sample volume. Testing results are displayed in Figure 3. For the purposes of this study,
 277 samples are considered to have failed in wetting-drying and freezing-thawing testing upon splitting and/or complete
 278 disaggregation. In addition, samples may be discontinued should volume measurements become inaccurate due to
 279 soil-cement loss (ASTM D559-559M, 2015). For the traditional binders, a minimum dosage of 10% was required

280 to maximise the chances of samples surviving all 12 testing cycles. Samples stabilised with CEM-I survived the 12
 281 cycle testing period with a gradual reduction in volume, whereas 10% lime samples only survived 3 cycles
 282 exhibiting larger volumetric losses. According to Sargent et al. (2013), most non-activated geopolymers fail after
 283 the first cycle. However, the 5 and 10% GGBS samples survived 2 and 3 cycles respectively.
 284 GGBS-NaOH dosages of 2.5 and 5% displayed similar disappointing performances to those observed for non-
 285 activated GGBS samples. However, impressive performances were recorded for specimens containing a dosage of
 286 7.5 or 10%, as they displayed negligible changes in volume or moisture content over the 12 cycles. Whilst the
 287 Lanton + 7.5% and 10% GGBS-NaOH samples demonstrate higher levels of durability against wetting-drying
 288 compared with Lanton + 10% GGBS samples, the decreasing water content of such non-activated samples after
 289 developing latent hydraulicity inhibits the hydration of cementitious gels and therefore long-term strength
 290 development.



291

292

293 **Figure 3:** Volumetric (a-b) and moisture content (c-d) changes experienced by all stabilised Lanton alluvium mixtures during
 294 wetting-drying testing.
 295

296 **4.2.2 Freezing-thawing**

297 Freezing-thawing testing was conducted to assess the ability of the binder mixtures to resist repeated freezing-
 298 thawing cycles by measuring changes in sample volume, moisture content and soil-cement losses. In the UK,
 299 freeze-thaw occurs up to depths of 2m, subject to variations between locations due to in-situ soil and vegetation
 300 conditions (Clarke and Smethurst, 2010). Given the grading of Lanton alluvium and that water may easily percolate
 301 through the soil; shrinkage, swelling and physical weathering are likely to occur within the shallow sub-surface.

302 This raises concerns as the binder within DDSM columns must resist such processes. Albeit the ASTM
 303 D560/560M procedure may be considered extreme for simulating typical freeze-thaw conditions in the UK, the
 304 results are conservative and will prove useful when assessing similar alluvial soils in other countries with harsher
 305 winters. Results from the freezing-thawing testing are presented in Figure 4.

306 Using a dosage of 5% for lime and CEM-I was too low to produce any marked strength or durability enhancements,
 307 as samples failed after their first cycle of testing. Although the 10% lime specimens only survived 2 cycles with a
 308 significant volumetric reduction, the 10% CEM-I samples survived all 12 cycles. However, the samples did
 309 experience a volumetric reduction of up to 60%. NaOH activation of GGBS was essential to promote pozzolanic
 310 conditions and ultimately ensure that samples survived beyond 3 – 10 cycles. The 10% GGBS-NaOH mixture was
 311 the most effective binder tested, followed by the 7.5 % dosage, as they provided high levels of resistance to harsh
 312 freezing-thawing cycles. This was confirmed by specimens surviving all testing cycles and the negligible changes
 313 in volume or moisture content.

314

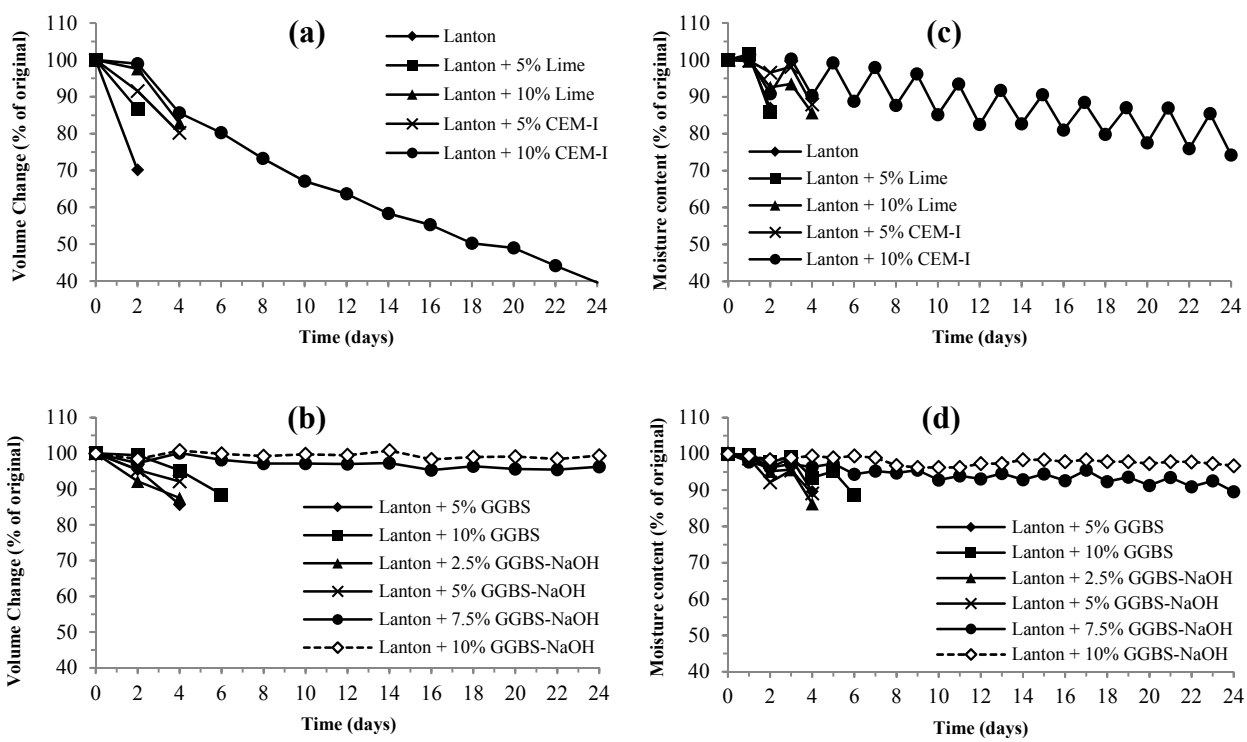
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316

317 **Figure 4:** Volumetric (a-b) and moisture content (c-d) changes experienced by all stabilised Lanton alluvium mixtures during
 318 freezing-thawing testing.

319

320 To summarise, GGBS-NaOH at dosages >7.5% (particularly 10%) produced the best durability performances for
 321 both wetting-drying and freezing-thawing, surpassing that of 10% CEM-I. NaOH activation proved very effective
 322 in producing dense high strength samples, which possessed low permeabilities and were thereby deemed less likely

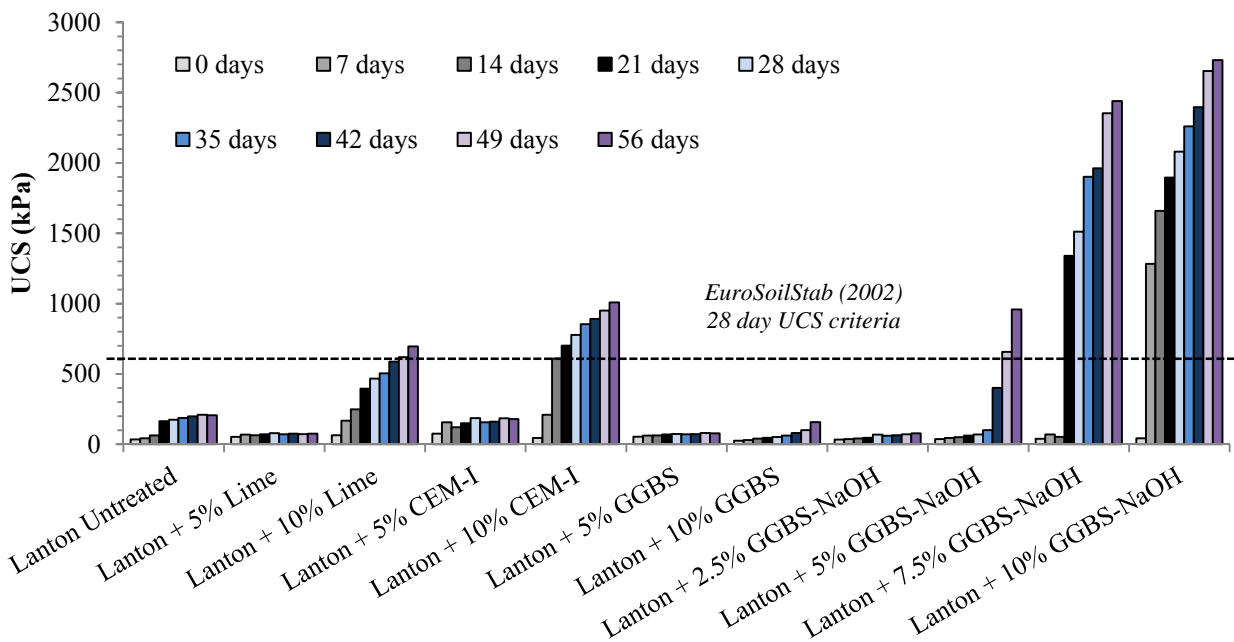


323 to experience water absorption and subsequent sample deterioration through shrinkage, swelling and physical
 324 weathering.

325
 326 4.3 Compressive strength

327 A summary of the average maximum compressive strengths achieved by the various stabilised mixtures is
 328 presented in Figure 5. With a few exceptions, sample strengths increased over the 28 day curing period for all
 329 mixtures. However, the degree of strength development observed within samples varies significantly between
 330 binder dosages. Using 2.5 or 5% for any binder type proved unsuccessful in producing 28 day UCS values of
 331 300kPa required by EuroSoilStab (2002).

332 The strengths recorded for 5 and 10% GGBS were also low, reaching 56 day strengths of 77kPa and 157kPa
 333 respectively; both failing to match the strengths achieved by either lime or CEM-I at equivalent dosages and curing
 334 periods. These samples were characterised by highly ductile behaviour at failure. Hence, alkali activation was
 335 necessary to produce any considerable strength development. Four dosages (2.5, 5, 7.5 and 10%) of NaOH
 336 activated GGBS were assessed to identify the most sustainable and optimum dosage in terms of strength gain.
 337 There was no advantage in using either the 2.5 or 5% dosages due to the strengths achieved for each curing period
 338 (70kPa after 28 days) closely resembling the performance of untreated Lanton alluvium. The 10% GGBS-NaOH
 339 samples exhibited the highest and most rapid strength enhancements with curing, with the effects of NaOH
 340 activation first being seen after 14 days curing. After 28 and 56 days curing, impressive UCS values of 2.08 and
 341 2.73MPa were recorded, respectively.



342
 343 **Figure 5:** UCS performances for all binder mixtures and dosages used to stabilise Lanton alluvium after all curing periods.

345 Although the 7.5% GGBS-NaOH binder dosage produced lower strengths than the 10% dosage after 7 and 14 days;
346 significant strength enhancements initiated after 21 days curing, producing UCS values of 1.3 – 1.4MPa which
347 exceed strengths for samples stabilised with 10% lime or CEM-I by factors of 3 and 2, respectively. The strengths
348 also met EuroSoilStab (2002) 28 day 300kPa requirement. The behaviour of the 7.5 and 10% GGBS-NaOH
349 samples after 28 days was brittle upon failure. The 10% GGBS-NaOH samples exhibited higher levels of brittle
350 behaviour due to their higher strength, even after 7 days curing. This contrasts with the more ductile behaviour
351 observed for the other mixtures tested after 7 days.

352 Using NaOH with GGBS at dosages $\geq 7.5\%$ was necessary to produce significant strength gains. For the 5%
353 dosage, strength development commenced after 42 days – reaching comparable 56 day strengths to 10% CEM-I.
354 However, such delayed strength gains are unacceptable, as construction specifications require strength
355 enhancements after 28 days (Hansson et al., 2001).

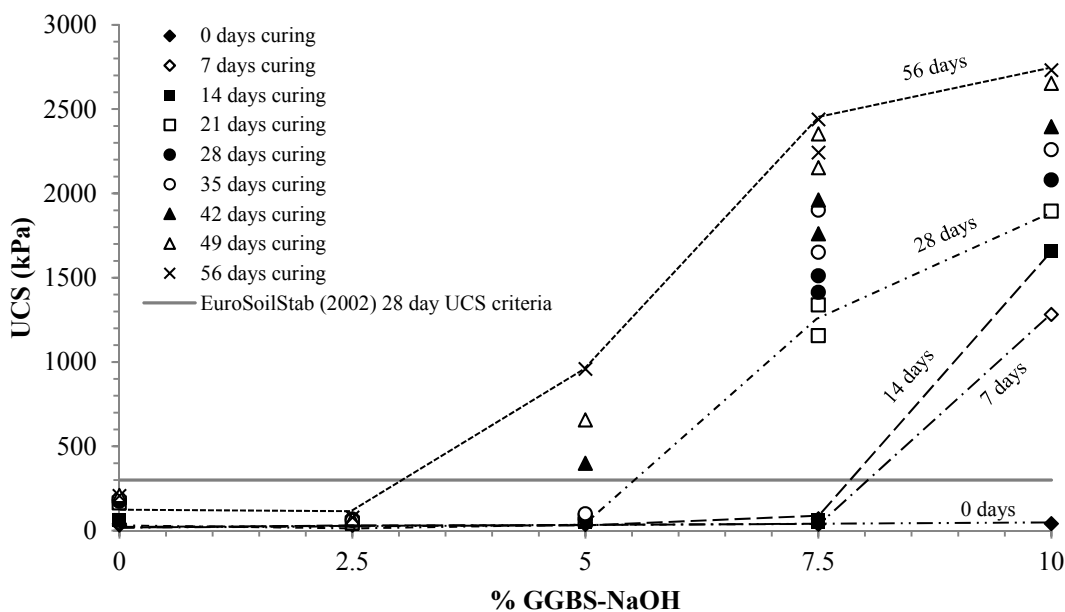
356 Given the Lanton alluvium's low permeability, there are two factors which may explain why such impressive
357 strength gains were achieved within stabilised samples. Firstly, the activated GGBS binder was thoroughly mixed
358 into the soil to achieve a high level of homogeneity; thereby allowing hydration and pozzolanic reactions to occur
359 throughout samples and produce a wide distribution of cementitious gels. The second factor may be attributed to
360 the even distribution of water throughout samples, which is partly owed to the thorough sample mixing and the
361 soil's particle size distribution. The soil's sand and silt fractions provide hydration reactions with better access to
362 water for cementitious gel formation compared with soils containing higher clay contents.

363 GGBS-NaOH dosage influences strength development as can clearly be seen on Figure 6. Whilst further testing is
364 required to more definitively identify the most optimum dosage for use in stabilising Lanton alluvium, Figure 6
365 uses the strength results achieved with increasing curing time for the four GGBS-NaOH dosages in an attempt to
366 approximate an optimum dosage. The performance of this new binder also surpassed that for the CEM-I and lime
367 binders. According to the 28 day strength development trend line presented in Figure 6, there is an indication that
368 the GGBS-NaOH dosage which satisfies the EuroSoilStab (2002) strength criterion would be approximately 5.5%.
369 However, this dosage would be insufficient to produce the 28 day strength of 778kPa recorded for 10% CEM-I;
370 whereby an approximate GGBS-NaOH dosage of 6.5% would be required. Thus, to satisfy both strength criteria, an
371 optimum GGBS-NaOH dosage of 6.5% would be envisaged for inclusion within the Lanton alluvium.

372 Moisture content and porosity also influence strength development, whereby mixtures with higher moisture
373 contents have lower calcium concentrations (pertaining to GGBS) for cement formation; resulting in lower

374 strengths. Consoli et al. (2011) suggested porosity also influences strength by modifying the number of contact
 375 points between soil particles; whereby delayed strength gains are anticipated within soils of higher porosities and
 376 those which contain less cement and therefore a less dense network of cementitious gels.

377



378

379 **Figure 6:** Relationship between unconfined compressive strength and GGBS-NaOH dosage, with trend lines shown for 0, 7,
 380 14, 28 and 56 days curing.

381

382 Suctions may have been generated within UCS samples during their preparation, which may have resulted in higher
 383 sample strengths than anticipated. DDSM treatment of sands using CEM-I has been adopted successfully world
 384 wide for many years; whereby high strengths are generally achieved early (within 14 days) with very limited long-
 385 term (months-years) strength development. For more clayey soils, these are generally required to have 10 – 50 %
 386 clay content to have sufficiently adequate CEC and surface area characteristics to promote conditions required for
 387 long-term strength improvements through DDSM (Tutumluer, 2012). Conversely, higher clay contents reduce the
 388 soil's permeability; as does the addition of GGBS. Stabilising soils with clay contents >30% may not be as
 389 successful compared with soils containing <30% clay, as it would reduce the volume of water which hydration and
 390 long-term pozzolanic reactions could utilise to form C-S-H/C-A-H gels (Hughes et al., 2011).

391

392 4.4 Moisture content

393 To assess whether samples contained adequate supplies of water to allow for continued hydration and cementitious
 394 bond formation, water content was determined according to BS 1377: Part 2 (BSI, 1990). Most samples had water

395 contents of 22 – 25%. According to Table 4, each mixture showed slight reductions in moisture content over 28
 396 days. Negligible reductions were recorded within all non-activated samples. For GGBS-NaOH samples, moisture
 397 content reduced by 1 – 1.8%; whereby reductions >1.4% were observed for those containing dosages >7.5%. The
 398 most significant reductions (up to 5.25%) were observed in samples stabilised with a 10% lime and CEM-I.

399 Sub-samples were retained after UCS testing at 28 days and tested according to BS 1377: Part 3 (BSI, 1990).
 400 Average pH values recorded for Lanton alluvium ranged between 6.7 – 7.2 over 56 days; thereby indicating the
 401 need for an alkali activator or a high alkaline binder. Stabilisation using 5 – 10% CEM-I or lime produced pH
 402 values >11.2 and were sustained for the 56 day testing period.

403

404 **Table 4:** Average changes in water content observed over 28 days for all untreated and stabilised mixtures.

Soil – Binder Mixture	Water content (%) with increasing curing time		Reduction in water content (%)
	0 days	28 days	
Lanton	24.6	23.9	0.7
Lanton + 5% Lime	24.1	23.3	0.8
Lanton + 10% Lime	25.1	21.6	3.5
Lanton + 5% CEM-I	24.2	22.7	1.5
Lanton + 10% CEM-I	24.5	19.3	5.3
Lanton + 5% GGBS	24.8	24.1	0.7
Lanton + 10% GGBS	24.9	24.6	0.4
Lanton + 2.5% GGBS-NaOH	24.6	23.5	1.0
Lanton + 5% GGBS-NaOH	25.6	24.5	1.1
Lanton + 7.5% GGBS-NaOH	25.1	23.6	1.5
Lanton + 10% GGBS-NaOH	24.7	22.9	1.8

405

406 Moisture content reductions 1 – 1.8% correspond with samples which achieved high UCS values and durability
 407 performances; (e.g. 7.5 – 10 % GGBS-NaOH) suggesting that cementitious bonds formed through hydration and
 408 pozzolanic reactions. This implies that all sample mixtures contained sufficient volumes of water for cementitious
 409 bonding reactions during and beyond 56 days.

410 In line with Duxson et al. (2007), a relationship may exist between decreasing moisture content and strength
 411 enhancement when Lanton alluvium is stabilised with lime or GGBS-NaOH. As soil water is consumed during the
 412 dissolution of geopolymetric reactions and the breakdown of solid aluminosilicates to give silicate and aluminate
 413 species through alkaline hydrolysis (Duxson et al. (2007)); it is unsurprising that GGBS-NaOH samples showed
 414 greater reductions in water content compared with non-activated GGBS. By increasing the GGBS-NaOH dosage
 415 within the alluvium, this inherently increases the calcium concentration within the stabilised material due to the
 416 GGBS. This would result in water within the soil being utilised at an accelerated rate to form cementitious gels and
 417 therefore achieving higher strength gains over 28 days compared with using lower GGBS-NaOH dosages (i.e.

418 <5%). This corroborates with the strength results presented in Figure 5 and the degrees of moisture content
419 reduction within the four GGBS-NaOH dosage sample mixtures.

420 Considering that Lanton alluvium has a natural water content of 25%, there is potentially scope for further
421 hydration and pozzolanic reactions beyond 56 days, and therefore further water consumption and cementitious gel
422 development. In the long term, Lanton alluvium stabilised with binder dosages <7.5% may outperform the
423 performances of stabilised soils containing water contents of <25%. Locat et al. (1990) attributed such behaviour to
424 solutes being able to migrate through the soil's pore spaces more easily. This may be apparent for the 5% GGBS-
425 NaOH sample, which started to display encouraging strengths after 42 days.

426 Locat et al.'s (1990) research implies that stabilising Lanton alluvium with 10% GGBS-NaOH closely resembles
427 rapid strength gain behaviour for soils with low water contents. Using 7.5% GGBS-NaOH showed slower rates of
428 strength development, with 28 day strengths of 1.3MPa. Comparing this behaviour with Locat et al.'s (1990)
429 model, it corresponds well with slower rates of strength development typically observed for soils with high water
430 contents. For 5% GGBS-NaOH, negligible strength developments were observed until 42 days. These findings
431 prove that further study is required over a longer time period to gain a better understanding of the progressive
432 strength development of GGBS-NaOH stabilised Lanton alluvium at various dosages.

433

434 4.5 Compressibility and stiffness

435 Based on the relatively high strength and durability performances of the GGBS-NaOH binder, compressibility
436 testing was focussed on the Lanton alluvium when untreated and stabilised with GGBS-NaOH. In the UK, Network
437 Rail standard RT/CE/S/065 Issue 4 2014 "Examination of Earthworks" Risk Matrix (Network Rail, 2014) considers
438 railway embankment height ranges of: a) <3m b) 3 – 6m and c) 6 – 10m. Most failures occur 3 – 6m high
439 embankments with slope angles >25 degrees. On the UK's railway network, embankment heights are known to
440 typically vary between 3 and 10m. However, there are locations where embankment heights are known to exceed
441 the aforementioned height range, whereby embankments reach up to 30m high. Vertical effective stresses
442 associated with 30m high embankments, assuming embankment fill unit weight of 20kN/m³ and worst case shallow
443 groundwater levels (5mbgl) may be as high as 300 – 400kPa. Hence, m_v values recorded for each sample mixture
444 under vertical stresses up to 400kPa during compressibility testing were considered. It is under the vertical stresses
445 of less than or equal to 400kPa where specimens also experience the most compressional strain during testing;
446 hence why m_v values obtained for vertical stresses (σ_v) of 800 – 1600kPa have not been included.

447 Compression curves for the soil in its untreated and GGBS-NaOH stabilised states after 0 and 28 days curing are
448 displayed in Figure 7. The coefficients of compressibility (m_v) values obtained for each sample after 0 and 28 days
449 curing are given in Table 5.

450 The compression curves in Figure 7a illustrate a reduction of 0.23 in the initial void ratio for the remoulded Lanton
451 alluvium, compared with its undisturbed counterpart. This reduction can be attributed to the removal of the soil's
452 natural structure, which provides a degree of additional strength to the material due to fabric and/or inter-particle
453 bonding as a result of the soil's geological history – i.e. depositional and post-depositional processes (Gasparre and
454 Coop, 2007). A more significant reduction in initial void ratio can be seen in Figure 7a for all 28 day cured
455 stabilised samples compared with the undisturbed (untreated) Lanton alluvium. This is an inherent result of
456 densification due to the nature of the DDSM process; whereby the orientation of the auger mixer blades provides a
457 degree of compaction. Additionally, by introducing a fine grained (silty) cementitious GGBS binder into the
458 alluvium, this alters the PSD curve for the material and infills void spaces within the soil – thereby having the
459 effect of reducing its initial void ratio. However, the increase in material stiffness due to void ratio reduction is
460 small compared with that provided by the formation of cementitious gels.

461 After 0 days, all stabilised mixtures experienced void ratio reductions of 0.1, which contrasts with untreated
462 samples whose void ratios decreased by 0.25 and 0.4 for remoulded and undisturbed samples, respectively. Given
463 that introducing a binder via DDSM disturbs the soil, it is unsurprising that the initial void ratios of stabilised
464 samples were similar to the remoulded soil. After 28 days curing, samples containing 2.5 and 5% GGBS-NaOH
465 exhibited slightly improved consolidation behaviour compared with that observed after 0 days curing, as proven by
466 the compression indices (C_c) and swelling indices (C_s) values in Table 5. Figure 6b shows samples containing 7.5
467 or 10% GGBS-NaOH displayed a significantly stiffer response after 28 days. C_c values for these mixtures
468 decreased considerably compared with their 0 day values. C_s values for all four dosages were almost identical. C_c
469 values for 7.5 and 10% dosage samples are at least one third of the values obtained for samples containing 2.5 or
470 5% binder, 9 and 20 times smaller than the C_c values obtained for the remoulded and undisturbed soil, respectively.

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479 **Table 5:** m_v , C_c and C_s values for treated and untreated Lanton alluvium after 0 and 28 days.

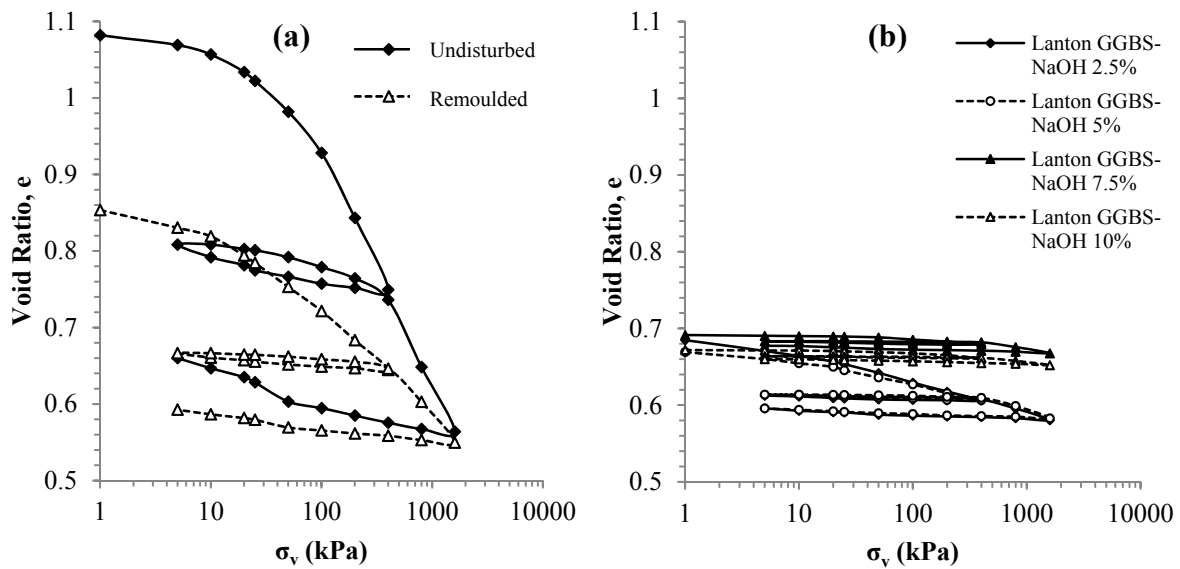
Soil – Binder Mixture	Coefficient of Compressibility (m_v), m^2/MN								Compression index (C_c)		Swelling index (C_s)	
	0 days				28 days				0 days	28 days	0 days	28 days
	50kPa	100kPa	200kPa	400kPa	50kPa	100kPa	200kPa	400kPa				
Lanton remoulded	0.703	0.359	0.221	0.116	0.703	0.359	0.221	0.116	0.132 0.309	0.013 0.030	0.015	0.004
Lanton undisturbed	0.799	0.542	0.440	0.255	0.799	0.542	0.440	0.255				
Lanton + 2.5% GGBS-NaOH	0.238	0.074	0.098	0.055	0.276	0.153	0.073	0.036	0.065	0.039	0.015	0.004
Lanton + 5% GGBS-NaOH	0.231	0.150	0.071	0.058	0.230	0.109	0.070	0.029	0.087	0.034	0.011	0.004
Lanton + 7.5% GGBS-NaOH	0.222	0.142	0.071	0.046	0.025	0.035	0.016	0.011	0.077	0.016	0.011	0.004
Lanton + 10% GGBS-NaOH	0.214	0.133	0.071	0.034	0.025	0.019	0.015	0.012	0.055	0.014	0.009	0.003

480

481 The m_v results complement the compression curves, whereby all of the binder dosages reduced the compressibility
482 of the untreated Lanton alluvium. The initial m_v values of the untreated undisturbed and disturbed alluvium ranged
483 between 0.7 – 0.8 m^2/MN , which is typical of normally consolidated alluvium and therefore highly compressible
484 (Tomlinson, 2001). The 0 day m_v values for each dosage ranged between 0.21 m^2/MN for vertical stresses (σ_v) of
485 50kPa and 0.03 m^2/MN for $\sigma_v=400kPa$. These values indicate that each mixture had a medium to very low
486 compressibility. For dosages of 2.5 and 5%, negligible changes in m_v values were achieved after 0 and 28 days
487 curing. However, there was a significant reduction in m_v values after 28 days for samples containing 7.5 or 10%
488 GGBS-NaOH, where the effect of cementation is apparent. Considerably lower m_v values of 0.01 – 0.03 m^2/MN
489 and 0.01 – 0.02 m^2/MN were calculated for samples containing 7.5 and 10% binder dosages, respectively; and are
490 deemed to be of very low compressibility.

491 These results suggest that the most effective dosage of the GGBS-NaOH binder in reducing the initially high
492 compressibility of untreated Lanton alluvium was 10%. However, the 7.5% dosage was equally as effective when
493 samples were subjected to $\sigma_v > 200kPa$. Hence, there is little benefit in using a dosage of 10% over 7.5%, given their
494 similarly small reductions in void ratio after two loading-unloading cycles.

495



496
497 **Figure 7:** Oedometer compression curves of Lanton alluvium in (a) its untreated remoulded and undisturbed state, and (b)
498 treated with 2.5-10% GGBS-NaOH after 28 days curing. Note: the curves are based on averages taken from triplicate testing.

499
500 The undisturbed soil possessed a fairly high level of sedimentation structure. Slight disturbance or increase in
501 loading up to 100kPa resulted in a sudden and considerable reduction in void ratio, and therefore significant
502 structural collapse. This highlights treatment is required to improve the soil's high levels of compressibility.

503 In summary, a significant improvement in compressibility and therefore stiffness was observed after 28 days for 7.5
504 and 10% GGBS-NaOH specimens; whereby m_v values decreased to $<0.03\text{m}^2/\text{MN}$ with increasing σ_v , indicating
505 very low levels of compressibility. Using $>7.5\%$ GGBS-NaOH within Lanton alluvium produced brittle failures.
506 This behaviour could be unfavourable under dynamic loading conditions due to passing rail traffic and earthquakes.
507 However, the strengths achieved were lower and more favourable than those recorded by Sargent et al. (2013) for a
508 GGBS-NaOH- Na_2SiO_3 stabilised artificial alluvium.

510 5.0 Sustainability assessment

511 New geopolymers must demonstrate commercial and practical viability as alternatives to CEM-I and lime. Per
512 Global Cement (2011), demonstrating GGBS-NaOH for use in DDSM on an industrial field scale is essential to
513 proving its practicality and for developing knowledge regarding the cost implications of non-equitable economies
514 of scale. Assurance of this binder's potential for commercialisation should be taken from the commercialisation of
515 similar alkali-activated GGBS-based geopolymer concretes ("E-Crete") in Australia (Global Cement, 2011).

517 5.1 Environmental impact

518 Although the use and practicality of cement and lime as binders is well proven in DDSM, their continued usage is
519 environmentally unsustainable. Cement and lime production have high carbon costs. Cement production

520 contributes up to 7% of the world's CO₂ emissions (McLellan et al., 2011), whereby for every ton of cement
521 produced 900 – 1100kg of CO₂ is emitted (EPA, 2005). An estimated 4,100 million tonnes of cement was produced
522 in 2015 (Statista, 2016). The cement and civil engineering industries are under significant pressure to reduce CO₂
523 emissions and contribute towards atmospheric carbon capture storage (CCS).

524 Using IBP's as alternatives or partial replacements to CEM-I or lime has real potential for reducing the
525 environmental impact of using cements and lime. According to Hanson (2014), GGBS has impressive
526 environmental sustainability credentials. Compared with the production of lime and CEM-I, it reduces embodied
527 CO₂ emissions by 2 million tonnes per annum, reduces primary energy usage during manufacture/processing by
528 2000 million kWh, saves 3 million tonnes of quarrying and reduces landfill by almost 2 million tonnes. GGBS has
529 advantages over other IBP's due to the quantity of CEM-I which it can replace within a given concrete or stabilised
530 soil, whilst achieving engineering performances comparable with pure CEM-I (Connell, 2014). 50 – 70% of CEM-I
531 may be replaced by GGBS whereas only 20 – 25% may be replaced by PFA. Whilst approximately 75% of CaO
532 within the CEM-I component of concretes is able to sequester atmospheric CO₂ via carbonation (DTI, 2005),
533 mixing other high-calcium binders including GGBS and steel slag within urban soils has recently proved to have
534 excellent atmospheric CO₂ sequestration potential in the UK (Sanna et al., 2012). GGBS and steel slag have a
535 theoretical global CO₂ potential uptake of approximately 80 million tonnes per year (Yan, 2015). CCS within urban
536 soils through the incorporation of such slag materials has recently been demonstrated by Washbourne et al. (2012)
537 on the Science Central site in Newcastle upon Tyne, UK. Washbourne et al. (2012) demonstrated that the top metre
538 of soil for the 10 hectare site has the potential to capture and store 64,000 Mg of atmospheric CO₂ in the form of
539 carbonate minerals, half of which was captured within 3 years at an annual rate of 325 Mg C ha⁻¹. Similar figures
540 were also calculated for similarly mixed quarry soils (Barrasford, UK); namely 300 Mg C ha⁻¹, which was observed
541 down to depths of three metres over ten years (Manning et al., 2013). Such differences in sequestration rates were
542 attributed to differing soil mineralogies; whereby urban soils at Science Central contained cementitious rubble
543 material (calcium silicates and calcium hydroxide) which produce greater rates of carbonation compared with
544 Barrasford quarry soil (Manning et al., 2013). Globally, construction and development site soils are considered able
545 to capture and store 290 million Mg of atmospheric carbon within soil matrices per year (Renforth et al., 2009).

546 However, strength gains within GGBS-stabilised soft alluvial soils require alkali activation. Most alkali materials
547 for industrial purposes are synthetically sourced with high costs and negative environmental impacts. Although the
548 use of NaOH in this paper promoted pozzolanic conditions and high strength gains within stabilised samples, its
549 production involves the electrolysis of brine, which is energy intensive and produces considerable CO₂ emissions.

550 During its production, there is also the risk of contaminating local aquatic environments. A more sustainable NaOH
551 source could be the by-product from existing chlor-alkali plants (Kumar et al., 2012).

552 Significant CO₂ emission reductions of up to 64% are estimated for Australian geopolymers compared with CEM-I
553 (McLellan et al., 2011). There are carbon footprint implications associated with the brine electrolysis (membrane)
554 technique used to produce NaOH. However, the quantities of NaOH required in this research were small (0.83 –
555 3.3% by dry weight or 12.4 – 49.4kg m⁻³) compared with the quantities of CEM-I typically used in more traditional
556 binder mixtures for DDSM treatment of soft soils. According to Topolnicki (2004), cement dosages of 120 – 300
557 kg m⁻³ may be used within most soils, with the exception of sludges, peats and organic silts/clays where dosages of
558 250 – 400kg m⁻³ are recommended.

559 The GGBS CO₂ reductions far outweigh the carbon footprint produced by the NaOH's manufacture. There is also
560 potential for GGBS-NaOH to sequester atmospheric carbon within the stabilised soil, which assists in driving the
561 binder towards commercialisation. Further work is required on the effects of carbonation on the long-term
562 durability, strength and mineralogical development of stabilised soft alluvial soils and how much atmospheric CO₂
563 such mixtures can capture.

564 5.2 Financial costs

565 Although the engineering performances observed for GGBS-NaOH meet EuroSoilStab (2002) strength criteria and
566 surpass those exhibited by CEM-I and lime, it is also crucial to demonstrate that its overall cost is financially
567 competitive. There are considerable stockpiles of GGBS in the UK, given the country's active pig iron and steel
568 manufacturing industry. This makes GGBS sustainable in the UK for use in geopolymers as stockpiles are
569 continuously replenished and thus avoids expensive overseas sourcing. GGBS also requires little post-production
570 processing (i.e. ground granulation) to make it suitable for mixing.

572 Solid NaOH is available as flakes, pellets or pearls and is commercially available in raw bulk production and
573 scientific forms, the difference being that the latter has been purified. Hence, whilst the cost of scientific NaOH is
574 much higher, its effectiveness as an alkali activator is equal to that of industrial grade. For laboratory studies
575 involving various chemicals, scientific NaOH is recommended as it is in a much purer form. For large scale
576 applications, raw bulk production NaOH is more financially feasible. Additionally, the electrical energy consumed
577 per ton of NaOH produced is 1.285 kWh, which is almost 60 times lower than that required per ton of CEM-I
578 (Madloul et al., 2011).

579 The international market prices of these materials at the time of writing (July 2014 US\$:£ currency exchange rates)
580 were £25 – 60 per ton of GGBS and £175 per ton of NaOH (Alibaba, 2014). By taking the lowest available prices,
581 this produces a significantly lower price of £75 per ton for the GGBS-NaOH binder when using the 2/3 GGBS : 1/3
582 NaOH ratio as used in this paper. Whilst this price is more expensive than the international market price of CEM-I
583 (£50 per ton), it is only marginally more expensive compared with the UK price of £70 per ton. In order to make
584 the GGBS-NaOH break even with CEM-I in the UK, its price would only need reducing by 7% which would
585 equate to a binder dosage of 5.5% within Lanton alluvium. This advocates that GGBS-NaOH is potentially a
586 financially more sustainable alternative to CEM-I. McLellan et al.'s (2011) study demonstrated that for a number of
587 NaOH-bearing geopolymers developed in Australia, the total financial cost of these mixtures was not much greater
588 than that for CEM-I. However their transportation costs were considerably higher than those associated with CEM-
589 I, due to the significant transportation distances between binder source and the treatment site.

590 Turner and Collins (2013) stated that for an Australian geopolymer concrete comprising PFA, NaOH, sodium
591 silicate and a superplasticiser, a carbon foot print reduction of 9% was measured against a comparable strength
592 CEM-I concrete. There is a confliction between the reduced carbon footprint estimates provided by McLellan et al.
593 (2011) and Turner and Collins (2013). However, the geopolymers assessed by Turner and Collins (2013) contained
594 considerable quantities of sodium silicate, which is expensive and has significant negative environmental impacts
595 (Habert et al., 2011). Thus, the reductions in the GGBS-NaOH carbon footprint compared with CEM-I is more
596 comparable with estimates provided by McLellan et al. (2011). Based on the available evidence, GGBS-NaOH has
597 a smaller carbon footprint and lower financial cost compared with CEM-I; thereby highlighting its potential as a
598 commercially viable alternative to CEM-I.

599

600 **6.0 Engineering Practicality**

601 A key factor to consider in assessing the potential for the GGBS-NaOH binder to be used instead of lime or CEM-I
602 for future DDSM projects is engineering practicality. The application of chemical treatment in stabilising soft soils
603 is generally limited to those characterised by low organic and sulphate contents and relatively high clay contents.
604 These criteria have been defined to ensure that cation exchange and long term cementitious reactions occur within
605 the stabilised material. Although the Lanton alluvium possessed unfavourably high moisture and silt-sand contents,
606 using a GGBS-NaOH binder dosage $\geq 7.5\%$ proved effective in producing high levels of strength and durability
607 within the soil post 21 days curing. The enhanced mechanical performances of the GGBS-NaOH stabilised Lanton

608 soil were at least 2 – 3 times better than those observed for samples mixed with traditional binders, whilst meeting
609 strength criteria defined by EuroSoilStab (2002).

610 Another component which needs to be considered in assessing the engineering practicality of the GGBS-NaOH
611 binder for use in DDSM is whether any modifications to existing plant and equipment would be required. GGBS
612 comes in the form of a dried fine powder and that NaOH pellets/flakes are solid particulates. Hence, it is
613 anticipated that the GGBS-NaOH binder could be substituted for CEM-I and lime with relative ease; whereby only
614 minimal plant and equipment modifications would be required to ensure that the efficiency of binder delivery
615 during deep mixing is maintained. However, the use of NaOH on site can pose some health and safety risks for
616 DDSM operatives such as skin irritation (dermatitis) and eye damage. Thus, operatives should wear the appropriate
617 personal protective equipment including disposable rubber gloves, safety goggles/glasses, disposable overalls and
618 dust masks.

619 Based on the aforementioned factors, the GGBS-NaOH binder potentially has an impressive level of engineering
620 practicality, exceeding that of lime, CEM-I and other waste-based binders. In addition to being used as a binder to
621 stabilise soils, it may also be used as a partial/total substitute to CEM-I in concrete mixtures. However, the binder's
622 engineering practicality in terms of soil stabilisation requires further research; whereby the performance of the
623 GGBS-NaOH binder needs to be assessed when incorporated within soils characterised by higher organic and
624 sulphate contents compared with Lanton alluvium.

625 **7.0 Conclusions**

627 Alkali-activated IBP's show great potential for use as replacements for CEM-I and lime in stabilising alluvial soils.
628 The strengths and stiffnesses achieved by GGBS-NaOH significantly improved with curing, particularly when
629 using dosages >7.5% by dry weight (>107kg m⁻³), which comfortably met or surpassed those exhibited by samples
630 stabilised with equivalent quantities of lime/CEM-I and met criteria defined by EuroSoilStab (2002). For any of the
631 binder mixtures tested, a dosage <7.5% is not recommended given the natural chemical and geotechnical properties
632 of Lanton alluvium, along with the aggressiveness of the ASTM's durability testing programmes, which may occur
633 regularly in some countries.

634 Some engineering specifications require high strengths and stiffnesses and such cases, using GGBS-NaOH would
635 be the most logical replacement for CEM-I or lime. However, in addition to engineering scenarios characterised by
636 static loads, there are those such as high-speed rail lines where the high-frequency dynamic loading of the ground
637 requires slightly lower stiffnesses to prevent brittle failures. In such cases, using GGBS-NaOH at lower dosages

638 would be preferred. NaOH is very effective in activating GGBS and producing long term high strength gains,
639 which proved to be equally as effective as the sodium silicate-sodium hydroxide activator used by Sargent et al.
640 (2013) in raising soil pH, promoting pozzolanic conditions and long term strength gains. The GGBS based binders
641 and solid NaOH activator pellets could be substituted for CEM-I and lime in DDSM with minimal modification to
642 pre-existing plant and equipment, or a reduction in the efficiency of mixing.

643 The dosage of GGBS-NaOH for use within soils will differ between DDSM projects. For soils more problematic
644 than Lanton alluvium, higher dosages would be required to achieve high strengths. Additionally, the ratio between
645 GGBS and NaOH within the binder will also require customisation, whereby higher NaOH concentrations would
646 be required to stabilise soils with a low pH. Care must be taken in designing the GGBS-NaOH binder mixture, as
647 high NaOH concentrations would result in the binder becoming less environmentally and financially sustainable.
648 The use of GGBS-NaOH has the potential of becoming a more sustainable alternative than the continued use of
649 lime and CEM-I; thereby promoting its commercialisation potential. Although the current UK and international
650 market prices for the GGBS-NaOH binder mixture are higher than CEM-I, the financial costs in terms of raw
651 materials, energy consumption, transport and the associated CO₂ emissions incurred by CEM-I production far
652 outweigh those incurred by GGBS-NaOH. Ultimately, this new binder aims to achieve a balance between the desire
653 for making cost savings and a desire for a binder with low embodied carbon.

654 The cost of any DDSM project involving alkali-activated mixtures can be quite high. It is common for geopolymers
655 components to come from various locations. The distances between sourcing plants and stabilisation sites can be
656 considerable. As modern transportation costs are high, careful planning must be conducted to minimise delivery
657 distances, helping geopolymers become more sustainable and competitive than lime or CEM-I.

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